

**The University of Iowa**  
**Department of Civil & Environmental Engineering**  
**SOIL MECHANICS 53:030**  
**Midterm Exam II**  
**(1 Hour)**

Fall 1997

Instructor: C.C. Swan

**Problem #1:** (50 points)

A wastewater treatment aeration tank of diameter 40m and gross weight 286.5 MN is to be constructed on the site shown below in Figure 1a. To construct the tank, 6m of the dense sand layer will be excavated, and the tank will be built as shown in Figure 1b. For the values provided in Figure 1:

- Compute the increased average vertical stress in the silty clay layer directly beneath the center of the tank.
- Calculate the ultimate consolidation settlements that will occur at the center of the tank due to compression of the silty clay layer.
- How long will it take for 90% of this consolidation settlement to occur? (Use one-dimensional consolidation theory to answer this question.) The non-dimensional time constant for 90% consolidation is  $T_{90} = 0.848$ .

Note: This figure is **not** drawn to scale.

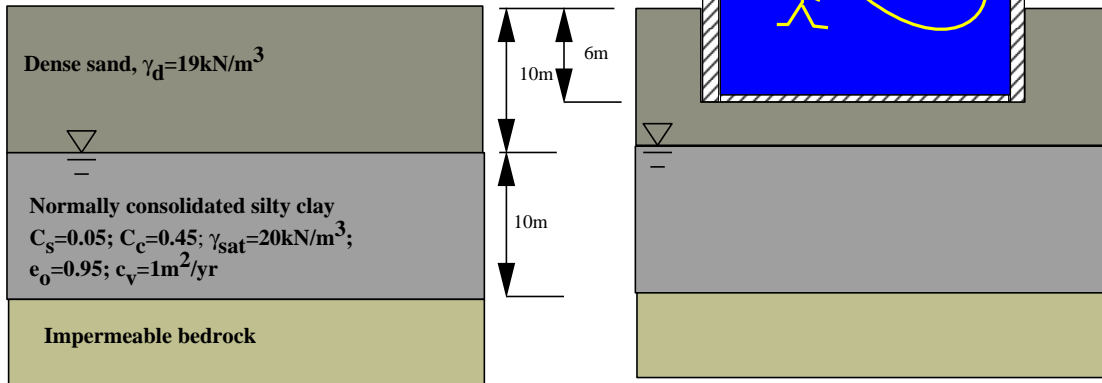


Figure 1a

Figure 1b

**Figure 1.** a) Existing site configuration; and b) Proposed configuration after construction.

**NOTE:** The increase in vertical stress below the center of a uniformly loaded circular flexible area is:

$$\Delta\sigma_v = q \left[ 1 - \frac{1}{[(R/z)^2 + 1]^{3/2}} \right]$$

where R is the radius of the loaded region and z is the depth below the center.

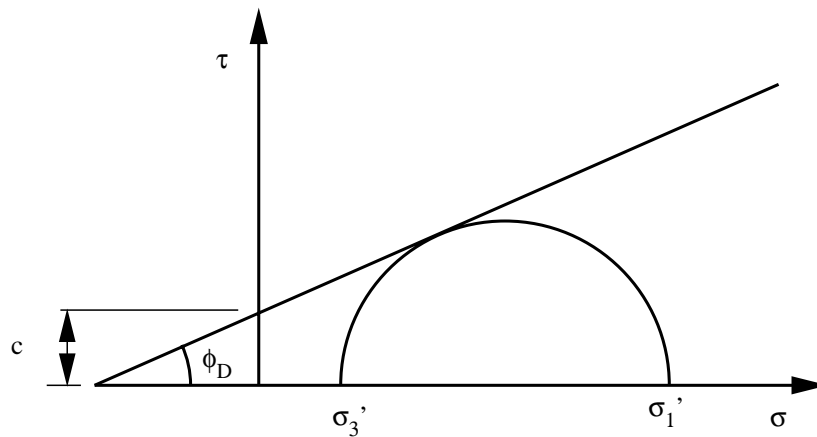
**Problem #2:** (50 points)

Two consolidated, drained (CD) triaxial compression tests were performed on over-consolidated silty soil samples removed from the field. The objective of the two tests was to measure the soil's cohesion  $c$  and drained friction angle  $\phi_D$ . The results of the two tests are as follows:

- Test 1: at failure,  $\sigma_1 = 287.94\text{kPa}$ ;  $\sigma_3 = 100\text{kPa}$ ;  $u = 100\text{kPa}$
- Test 2: at failure,  $\sigma_1 = 491.90\text{kPa}$ ;  $\sigma_3 = 200\text{kPa}$ ;  $u = 100\text{kPa}$

The tests were performed so slowly that the pore pressure  $u$  remained constant at  $100\text{kPa}$ . Therefore, the tests were **drained** triaxial compression tests.

- Using the Mohr's circle and Mohr-Coulomb failure geometry shown below in Figure 2, find a relationship between  $\sigma_1'$  and  $\sigma_3'$  at shear failure in terms of the cohesion  $c$  and friction angle  $\phi_D$ . **To receive credit, show all work.**
- Using this relationship, and the results of the two CD triax tests, solve for the cohesion  $c$  and drained friction angle  $\phi_D$  for the soil.
- During the triaxial compression tests on the soil samples, what would be the expected orientation  $\theta$  of the shear failure planes in the soil samples? (Use the Pole Method, and assume that  $\sigma_1$  is applied in the horizontal direction and  $\sigma_3$  in the vertical direction.) **Again, show all work to receive credit.**



**Figure 2.** Mohr-Coulomb failure envelope and Mohr's circle for consolidated-drained triaxial compression tests.

**Bonus Problem:** (15 extra points!!!)

- Identify three of the major assumptions used in deriving our one-dimensional soil consolidation model;
- Briefly explain how soil grain sizes (i.e. sands versus clays) affect observed shear strength behaviors in terms of drained and undrained responses.
- Using a sketch of "e versus  $\log(\sigma_v')$ ", briefly explain how and why pre-loading of sites is sometimes an effective method to reduce consolidation settlements under structural loads applied to clay/silt soils.